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ECCENTRICALLY LOADED STRIP FOOTING ON WEO CONTAMINATED SAND

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ABSTRACT
This paper presents an experimental investigation aimed at promoting a greater understanding of the behavior of strip footing on oil-contaminated sand. Contaminated sand layers were prepared by mixing sand with the waste engine oil (WEO) content of 1 % by weight of dried sand. A series of conditions, including uncontaminated cases, was tested by varying parameters such as the thickness of the contaminated sand layer and eccentricity of loading. The effects of eccentric loading on strip footing on uncontaminated and oil contaminated sand bed were studied in an experimental setup. The load-settlement behavior of foundation on uncontaminated and oil contaminated sand bed was also studied and compared in terms of BCR. The bearing capacity ratio is decreased with increasing the load eccentricity and the depth of oil contamination for contaminated sand bed.

KEYWORDS: Strip Footing; WEO; Eccentricity of Loading; BCR.

I. INTRODUCTION

Oil and gas are the most significant sources of energy worldwide, and their importance increases due to the ever increasing global demand for energy. However, the main drawback with this type of energy is the severe damage they caused to the environment due to the enormous amount of oil spills and leakage during their production. Air, water, and land are being contaminated for short-term benefits by industrial, petrochemical, construction, and sanitary activities. This pollution is usually caused by wars, vandalism, terrorism, and theft, but can also be caused by accidental leakage, oil spillage, corroding pipelines, transporting petroleum, human error during production and separation process. Intentionally or unintentionally, oil spill contamination impacts the physical and chemical properties of the surrounding sand.

Ghaly (2001) performed direct shear tests on oil-contaminated sands which showed a reduction in angle of friction with an increase in the oil percentage. Shin et al. (2002) reported a significant reduction in angle of friction with oil contamination. According to Ratnaweera and Meegoda (2006), the shear strength of granular soil decreases with an increase in pore fluid viscosity. Mashalah et al. (2007) carried an extensive laboratory testing program which shows that oil contamination induces a reduction in the permeability and strength of all soil samples. Meegoda and Ratnaweera in 1994 examined the compressibility of contaminated fine-grained soils by performing consolidation tests. Their finite-element analysis showed that the settlement of the foundation increases due to oil contamination. Al-Sanad et al. in 1995 and Al-Sanad and Ismael in 1997 found that oil contamination leads to decreased permeability and strength. Vesic in 1973 found that the the angle of friction and bearing capacity factor N_γ got reduced due to oil contamination.

Oil spills during transportation on the land or during oil drilling processes happen by accidents in most cases. When oil spills, soils might be contaminated by the leakage. Land contamination is not only harmful for the subsurface water aquifers but is also a detriment to the buildings and structures on it. Any change in the engineering properties and behavior of the soil strata may lead to a loss in the bearing capacity and an increase in the total or differential settlements of the foundation systems of structures. However, little information is available dealing with the effect of oil contamination of soil on the bearing capacity and settlement of shallow foundations. Therefore, the objective of the current study is to perform laboratory testing programs to determine the effects of

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oil contamination of sandy soil on the ultimate bearing capacity and settlement of a model strip footing. In addition, the comparison between uncontaminated and oil-contaminated sandy soil are studied.

II. MATERIALS AND METHODS

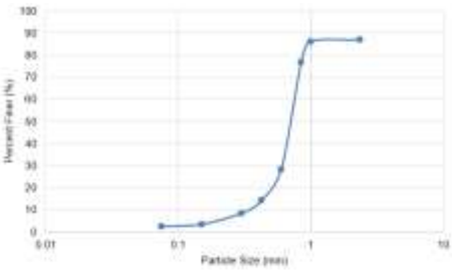
Test Tank and Test Footing

The apparatus used for the model tests is essentially a mild steel tank, 4 mm thick, 700 mm long, 400 mm wide, and 500 mm high with a steel base. The front long side was made of 20 mm thick glass, which allowed the sample to be seen during the preparation, to observe sand deformations during testing, and to minimize the friction between the tank wall and sand particles. The inside fixed walls of the tank are polished smooth to reduce friction with the sand as much as possible. Plain-strain conditions were considered for all model tests, therefore, the rigid footing was made of a rigid steel plate with a width of 100 mm, a thickness of 15 mm, and a length equal to the width of the tank to simulate a plain-strain strip footing. The grooves were made on the footing for centric and eccentric loading at e/B = 0.2 and 0.3 where e/B = ratio of eccentricity of loading to the width of footing. All tests were performed with the footing resting on the sand surface (D_f = 0.0). A rigid loading frame was used to apply the vertical load to the model strip footing. Two dial gauges were used to measure the settlement of the footing with an accuracy of 0.01 mm.

Testing Material : Kanhan Sand, Oil

Sand as a soil was used as fill material for the model tests. The particle size distribution was determined using the dry sieving method and results are shown in Fig. 1. Using the Unified Soil Classification System, the sand was determined to be poorly graded sand. All model tests were conducted at a relative density of 40 %. Direct shear tests were performed on specimens prepared at the same relative density used in the model tests with an estimated internal friction angle of 44°. The shear displacement during the tests was 1.0 mm/s. Some physical properties of the sand are given in Table 1.

Figure 1:



Grain-size Distribution Curve of Test Sand

Table 1. Physical Properties of Sand

Sr. No.	Physical Properties of Uncontaminated and Contaminated Sand		
	Properties	***Uncontaminated Sand***	***1 % Oil***
1	Specific Gravity (G)	2.70	2.60
2	γ_{max} (kN/m^3)	17.18	-
3	γ_{min} (kN/m^3)	15.88	-
4	Angle of internal friction, ϕ (DST)	44	35
5	Cohesion (kN/m^2)	0.0	18

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Sr. No.	Physical Properties of Uncontaminated and Contaminated Sand		
	Properties	*Uncontaminated Sand*	*1 % Oil*
6	Average grain size (D_{60}) (mm)	0.82	-
7	Effective grain size(D_{10}) (mm)	0.35	-
8	Coefficient of uniformity (C_u)	2.343	-
9	Coefficient of curvature (C_c)	1.472	-
10	I. S. Classification	SP	-

In experimental work, waste engine oil (WEO) from garage was used to contaminate the sand bed in the model tests. Oil added for contamination of sand worked as a softener which caused decrease in the friction between sand particles and strip bottom which further lead to decrease in bearing capacity of soil. Properties of waste of engine oil collected form Honda garage are shown in Table 2.

Table 2. Properties of Oil

Oil	Properties		
	Kinematics Viscosity (m^2/s)	*Density (kN/m^3)*	*Specific Gravity*
Waste Engine oil	45 x 10^{-6}	8.3	0.82

Preparation of Sand Bed

While testing on uncontaminated sand, sand rainfall technique was used to fill the tank. The height of fall to achieve the desired relative density was determined prior by performing a series of trials with different height of fall. The sand was poured in the tank by sand rainfall technique keeping the height of fall as 50 cm to maintain the constant relative density 40% corresponding bulk density 16.31 kN/m^3 throughout the test.

While testing on oil contaminated sand, uncontaminated sand at bottom was placed with the help of rainfall technique and then the oil contaminated sand layer for various depths, as per the depth of contamination to the width of footing ratio, was prepared by adding required percentage of waste engine oil in the sand in another container. This sand was placed inside the model box in 25 mm layers. For contaminated sand (moist sand), a raining technique for soil placement in the test tank was not suitable and did not provide uniformity in compaction. Therefore, the sand unit weight was controlled by pouring the pre-calculated weight of oiled sand into the box for each layer separately and then compacted to reach the required layer thickness. The length and width of contaminated sand layer was kept equal to the dimensions of an experimental tank and depth of contaminated sand layer was varied in an experimental investigations. Therefore, the unit weight of sand was controlled by pouring the precalculated weight of sand into the box for each layer separately, and the sand surface was levelled.

Experimental Procedure

The vertical load tests were conducted on a model pile foundation as per IS: 1888-1982 to evaluate the ultimate bearing capacity. The sand bed was prepared and the strip footing was placed on the sand bed. The hydraulic jack was attached to the horizontal member of loading frame with the help of plates and nut bolts. The proving ring was attached to the bottom of the jack. One metal rod was connected to the bottom threads of proving ring and rested directly on the groove made on the strip footing.

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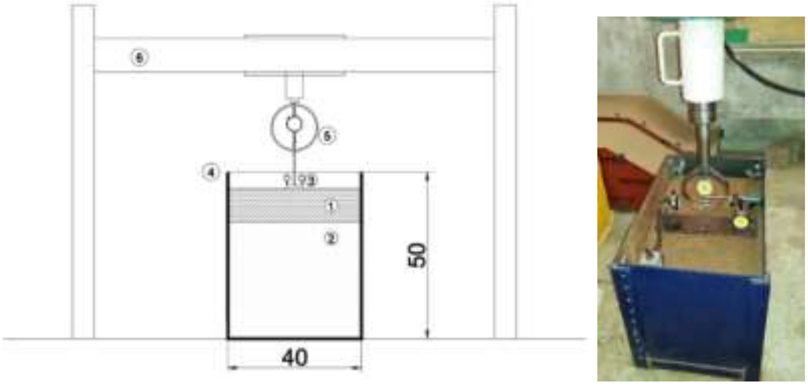
Dial gauges were placed on the strip footing to measure the vertical displacement on application of vertical load. Fig. 2 shows experimental setup.

1. Contaminated sand
2. Uncontaminated sand
3. Strip
4. Test tank
5. Proving ring
6. Reaction beam

Figure 2:



Schematic Diagram of Test Setup for Vertical Loading used for Experimental Investigations with Actual Experimental Setup

An experimental program was carried out to evaluate the effects of oil contamination on the performance of strip footing on sand bed. The research work aims to study the performance of strip foundation resting on uncontaminated and oil contaminated sand bed with respect to its various parameters under centric as well as eccentric loading. The theme of the project is to perform the model tests on strip foundation by varying its important parameter. Detailed experimental program of tests is given in Table 3.

Table 3. Details of Parametric Study in Experimental Investigation

Sr. No.	Details of Constant Parameters		Details of Variable Parameters	
	Parameters	**Description**	**Parameters**	**Description**
1	Type of Footing	Strip Footing	Eccentricity to Width of Footing Ratio (*e/B*)	*e/B* = 0, 0.2, 0.3
2	Type of Loading	Vertical	Depth of Contamination to Width of Footing Ratio (U/B)	U/B = 0.25, 0.5, 0.75, 1.0
3	Type of Soil	Kanhan Sand, Relative Density = 40 %		
4	Type of Oil and Percentage	Waste Engine Oil = 1%	-	-
5	Length of Contamination	Length of Contamination = 700 mm	-	-
6	Width of Contamination	Width of Contamination = 400 mm	-	-

III. RESULTS AND DISCUSSION

In this research, laboratory studies were conducted on model strip footing. The laboratory study would give an idea about the behaviour of strip footing on oil contaminated sand bed with loading eccentricity and depth of contaminated sand layer. During the experimental investigations, depth of contamination, eccentricity were augmented whereas the other parameters viz., dimensions of footing, type of soil, type of oil, length and width of contamination were kept constant. The model plate load tests were conducted on strip footing on uncontaminated and contaminated sandy soil bed prepared in experimental set-up and the load settlement curve was plotted for each test. The ultimate bearing capacities were determined from the load settlement curves.

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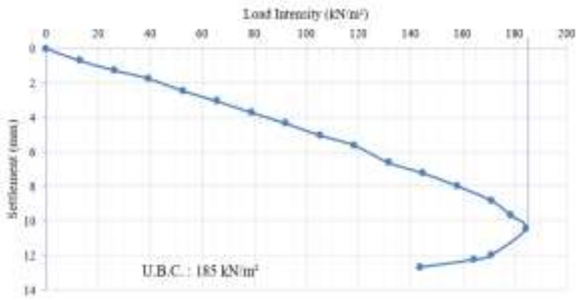
Then ultimate bearing capacities of footing on oil contaminated sand were compared with that of footing on uncontaminated sand in terms of bearing capacity ratio.

Performance of Strip Footing on Uncontaminated Sand

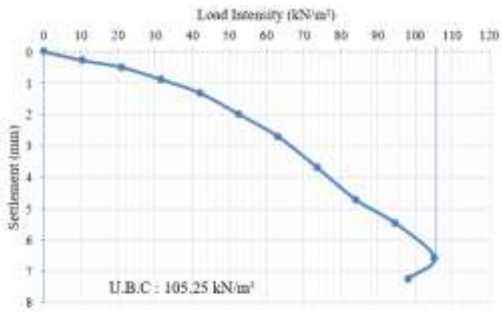
The results of the model plate load test are divided into two part according to loading condition viz., results for centric loading and eccentric loading. Both results are presented for different depths of contamination.

The load settlement curve for uncontaminated sand under centric loading condition is shown in Fig. 3 (a). The ultimate bearing capacity for strip footing is found to be 185.0 kN/m^2. For eccentric loading conditions i.e. for e/B = 0.2 and 0.3, the curves are shown in Fig. 3 (b) and Fig. 3 (c) respectively. The ultimate bearing capacities for strip footing under eccentric loading i.e. for e/B = 0.2 and 0.3, are found to be 105.25 kN/m^2 and 39.5 kN/m^2 respectively.

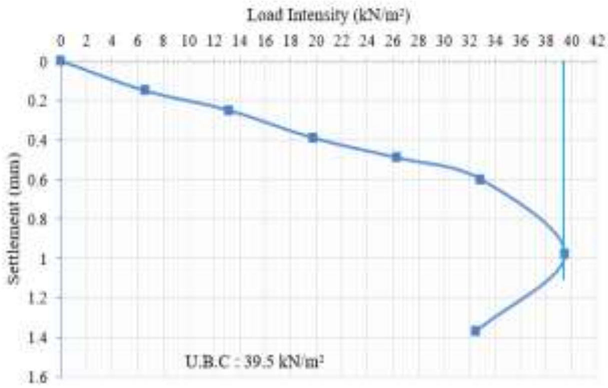
Figure 3:



(a) For Centric Loading (e/B = 0)



(b) For Eccentric Loading (e/B = 0.2)



(c) Eccentric Loading (*e/B* = 0.3)

Load Settlement Curve for Strip Footing on Uncontaminated Sand

Performance of Strip Footing on Oil Contaminated Sand under Centric Loading

The tests were conducted on model strip footing on prepared contaminated sand bed with 1 % percentage of waste engine oil (WEO). The load settlement curves for strip footing corresponding to different depths of contaminated layer of sand bed (U) with respect to width of the model strip footing are drawn to determine the ultimate bearing capacities (UBC). The load settlement curves, for *U/B* = 0.25, 0.5, 0.75, 1.00 under centric loading condition for strip footing resting on uncontaminated and WEO contaminated sand bed is shown in Fig. 4. The ultimate bearing capacities are then determined and presented in Table 4.

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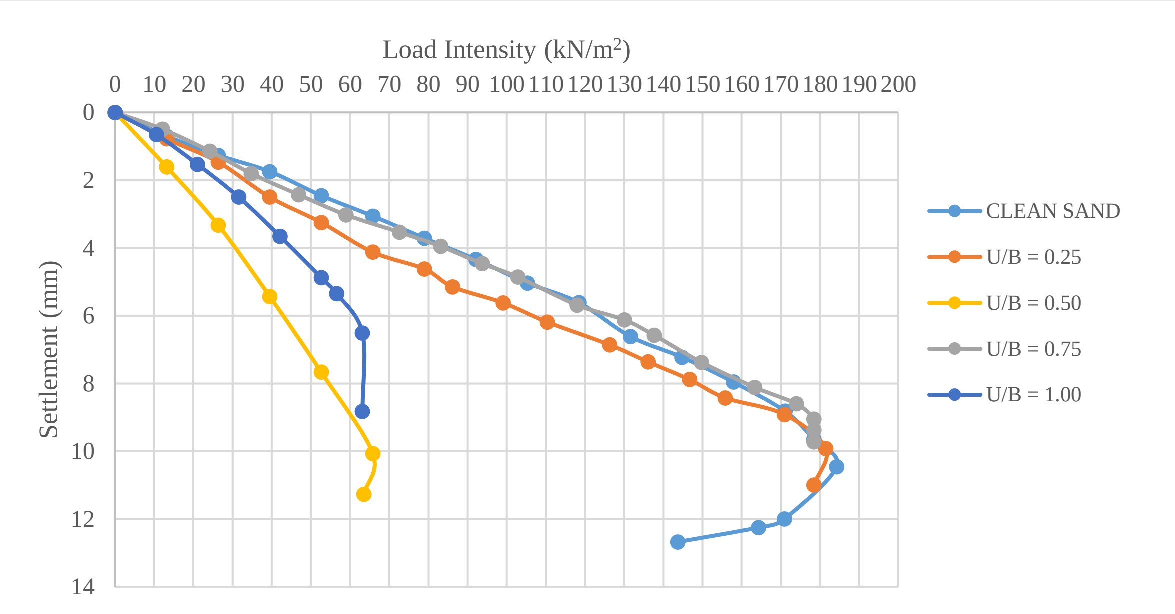
Figure 4:



Load Settlement Curves for Strip Footing on Uncontaminated and WEO Contaminated Sand under Centric Loading (e/B = 0)

Table 4. Ultimate Bearing Capacities and BCR for Strip Footing under Centric Loading

Sr. No.	WEO Percentage	*U/B*	UBC (kN/m²)	BCR
1	1	0.25	181.5	0.98
2		0.5	66.5	0.36
3		0.75	178.5	0.96
4		1.0	64	0.35

Performance of Strip Footing on Oil Contaminated Sand Bed Subjected to Eccentric Loading

The load settlement curves for eccentric loading for different depths of contaminated layer of sand bed (U) with respect to width of the model strip footing are drawn and UBC are determined. For different U/B ratios the load settlement behaviour for eccentric loading are determined and the ultimate bearing capacities are shown in the Table 5. The load settlement curves under eccentric loading condition for strip footing resting on uncontaminated and WEO contaminated sand bed is shown in Fig. 5.

Figure 5:

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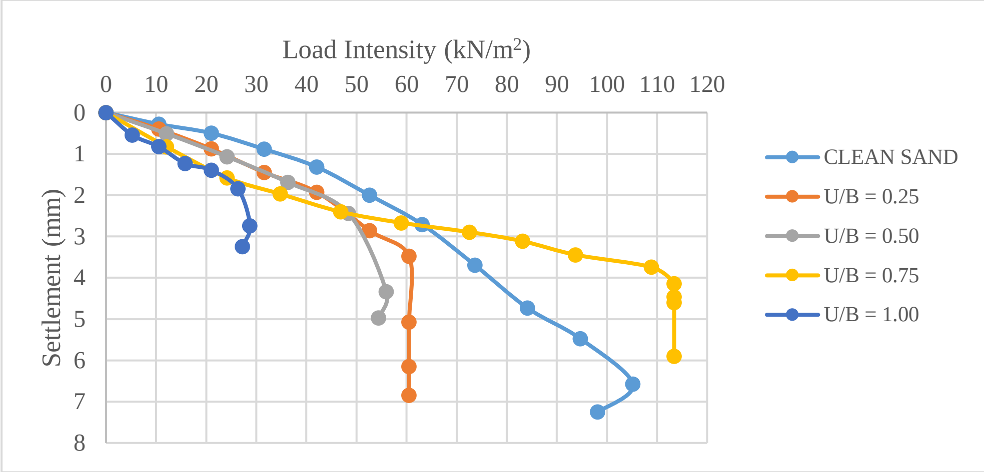
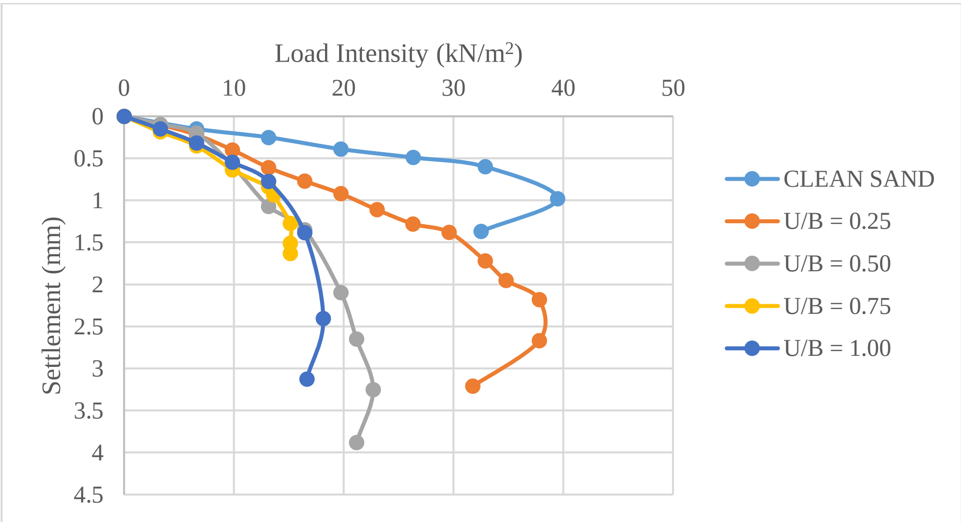


(a) For e/B = 0.2



(b) For e/B = 0.3

Load Settlement Curves for Strip Footing on Uncontaminated and WEO Contaminated Sand under Eccentric Loading (For WEO = 1 %)

Table 5. Ultimate Bearing Capacities and BCR for Strip Footing under Eccentric Loading (For WEO = 1 %).

Sr. No.	e/B	U/B	UBC (kN/m²)	BCR
1	0.2	0.25	61	0.58
		0.5	56	0.53
		0.75	113.5	1.10
		1	28.74	0.27
2	0.3	0.25	38.5	0.97
		0.5	22.7	0.57
		0.75	15.2	0.38

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		1	18.15	0.46

Summary and Discussion

In the first set, strip footing on uncontaminated sand under centric as well as eccentric loading was tested. Then effect of depths of contaminated sand bed was studied for centric and eccentric loading. The ultimate bearing capacities of foundation were determined form load settlement curves. The results for performance of strip footing on contaminated sand bed were compared with footing on uncontaminated sand in terms of bearing capacity ratio (BCR).

Bearing capacity ratio (BCR) is the ratio of ultimate bearing capacity of strip footing on contaminated sand bed under centric or eccentric loading to the ultimate bearing capacity of strip footing on uncontaminated sand bed under centric or eccentric loading.

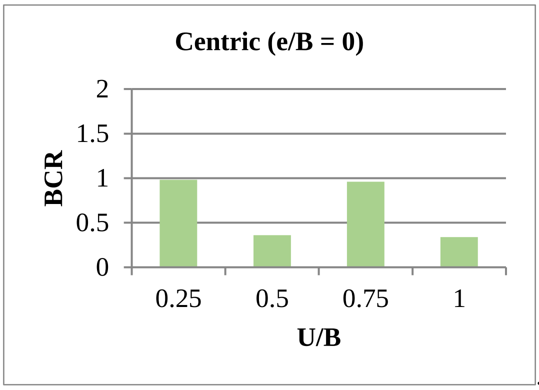
The effect of various parameters on the performance of strip footing on contaminated sand bed is discussed in following sections.

Effect of Depth of Contamination

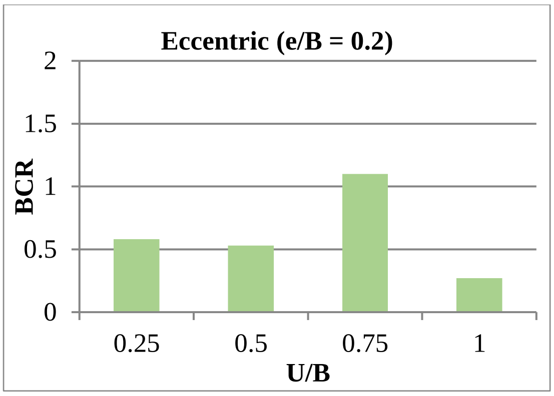
In the present study, four different depths of contaminated sand bed were adopted and model plate load test was carried out on strip footing resting on contaminated sand bed. Variation of BCR with depth of contaminated sand layer was studied for centric and eccentric loading with 1 % WEO.

Fig. 9. shows the variation in BCR of strip footing with different depths of contamination to the width of footing ratio (U/B) for centric and eccentric loading.

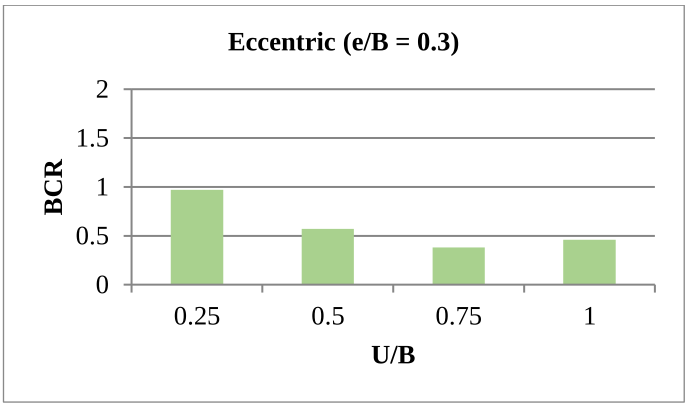
Figure 9:



(a) Centric



(b) e/B = 0.2



(c) e/B = 0.3

Effect of U/B Ratio on BCR for Strip Footing Subjected to Centric and Eccentric Loading

The bearing capacity ratio of strip footing subjected to centric as well as eccentric loading deceases with the increase in the depth of contamination.

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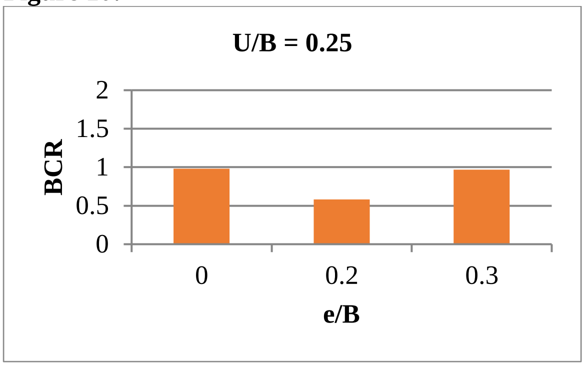
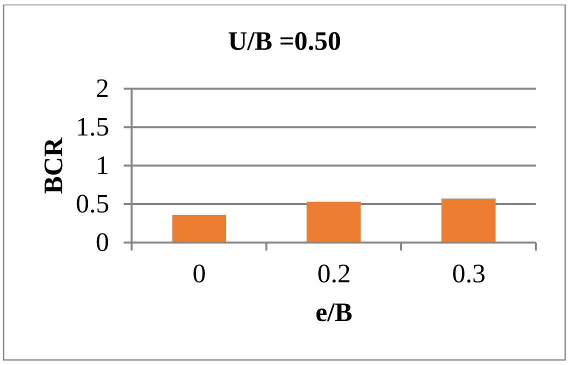
Effect of Eccentricity of Load

Tests were also conducted to study the effects of eccentricity of applied load on BCR of strip footing. Fig. 10 (a) to Fig. 10 (b) show the variation of BCR with respect to different e/B ratios in the form of bar charts for U/B = 0.25, 0.50, 0.75, 1.00 respectively.

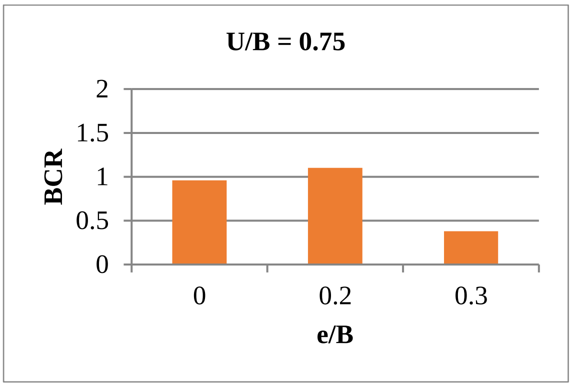
Figure 10:



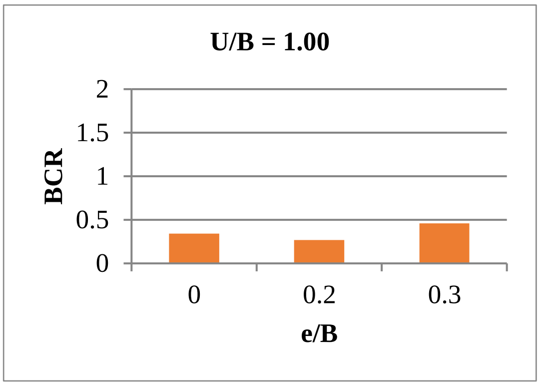
(a) For *U/B* = 0.25



(b) For *U/B* = 0.50



(c) For *U/B* = 0.75



(d) For *U/B* = 1.0

Effect of e/B ratio on BCR for Strip Footing

IV. CONCLUSION

From results, interpreted in the terms of BCR of strip footing, the following broad conclusions are drawn:

1. The bearing capacity ratio of strip footing subjected to vertically centric as well as eccentric loading decreases, in general, with the increase in the depth of oil contaminated sand bed.
2. The ultimate bearing capacity of strip footing on uncontaminated as well as contaminated sand bed decreases, in general, with the increase in the load eccentricity.

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